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R10

Set No. 1

III B.Tech II Semester Supplementary Examinations, November - 2017 DESIGN & DRAWING OF STEEL STRUCTURES

(Civil Engineering)

Time: 3 hours

Code No: **R32013**

Max. Marks: 75

Answer any ONE Question from Part – A and any THREE Questions from Part – B Use of IS 800-2007, IS-875 Part 3, Steel tables is to be permitted

<u>**PART – A**</u>(30M)

1 Design a suitable web splice for a plate girder at a section at which the bending [30M] moment is 5000kN.m and shear force is 1000kN. The plate girder is built-up from a web plate 2500mm deep×16mm and two ISA 200mm×150mm×15mm angles and a 600mm×16mm cover plate in each flange. The long legs of the flange angles are placed horizontally.

Draw:

(i) The elevation of the web splice showing all the details and

(ii) Cross-section of the plate girder at web splice.

(OR)

2 Design a gantry girder to carry an electric overhead travelling crane for an industrial [30M] shed to suit the following data:

(i) Crane load lifting capacity = 200kN

(ii) Weight of crane excluding trolley = 150kN

(iii) Weight of trolley = 75kN

(iv) Distance between gantry rails = 20m

(v) Minimum approach of crane hook = 1.2m

(vi) Distance between centres of crane wheels = 3m

(vii) Span of gantry girder =
$$6m$$
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(viii) Weight of rail section = $0.3 \frac{kN}{m}$

(ix) Height of rail section = 75mm Draw to scale the cross section and longitudinal section of the girder.

<u>PART – B</u>(3X15=45M)

3 a) A tie bar $120mm \times 10mm$ is to be connected to the other of size of $120mm \times 14mm$. [7M] The tie bars are to be loaded by a pull of 120kN. Find out the size of end fillets such that the stress in the end fillets is the same. Take the permissible stresses in the weldis $110 \frac{N}{M}$.

$$\frac{1013110}{mm^2}$$

b) An *ISA* 65mm×65mm×10mm carries a tensile load of 200kN, applied along its centroidal axis. This angle is to be welded to a gusset plate. Find out the lengths of side fillet welds required at the heel and toe of the angle.

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- A beam of span 8*m* carries a U.D.L of $20 \frac{kN}{m}$ over the whole length. Design the [15M] beam assuming that the compression flange is laterally restrained throughout the length. Take $f_y = 250 \frac{N}{mm^2}$
- 5 Design a compression member of two channels placed toe-to-toe. The length of the [15M] compression member is 12m and carries a load of 1500kN The width over the backs of channels is 450mm. The channels are connected by double lacing. Sketch the cross-section of the column.

[15M] Design the slab base for a column consisting of *ISHB* 300 at 58.8 $\frac{kg}{m}$ and carrying

- an axial load of 1000kN. Take allowable bearing pressure of concrete as $4 \frac{N}{mm^2}$.
- 7 Design a channel section purlin with and without sag bars for a trussed roof from the [15M] following data:

Span of roof =14mSpacing of purlin along slope = 2mSlope of roof truss = 1 vertical, 2 horizontal Spacing of truss = 3mWind load on roof =1200 N/m²

Vertical loads from roof sheets = $150\frac{4\pi}{m}$ www.hitstrates